

RAMEY KEMP & ASSOCIATES, INC. 4343 Cox Road

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January 26, 2017

Mr. Leon Hughes, AICP Spotsylvania County 9019 Old Battlefield Boulevard Suite 320 Spotsylvania, Virginia 22553 Phone: (540) 507-7220

Reference: River Road Residential - Traffic Impact Analysis

Dear Mr. Hughes,

Ramey Kemp & Associates, Inc. (RKA) has performed a Traffic Impact Analysis (TIA) for this proposed neighborhood, which consists of 39 single-family lots on the north side of Route 618 (River Road) between Route 620 (Spotswood Furnace Road) and Musket Ridge Lane. If approved, the neighborhood is expected to be built-out by 2018. Figure 1 shows the site location and study intersections.

The purpose of this letter report is to provide the following:

- Trip generation calculations
- Evaluation of turn lane warrants for the proposed connection to River Road
- Capacity analysis of the study intersections
- Sight distance study for the proposed connection to River Road
- Collision data analysis

Trip Generation

The trip generation potential of the neighborhood during a typical weekday, AM peak hour and PM peak hour was estimated using the methodologies published by the Institute of Transportation Engineers (ITE) Trip Generation Manual – 9th Edition. Table 1 summarizes the trip generation calculations.

Table 1 ITE Trip Generation - 9th Edition - Weekday

Land Use (ITE Land Use Code)	Size	Weel Daily 7 (vr	Traffic	AM Pea (vp	and the second second	PM Peak Hour (vph)		
		Enter	Exit	Enter	Exit	Enter	Exit	
Single-Family Homes (210)	39 homes	221	221	9	28	28	17	

Existing Roadway Conditions

Route 618 (River Road) is a two-lane minor collector with an average daily traffic (ADT) volume of approximately 2,400 vehicles per day, and a posted speed limit of 35 mph west of Spotswood Furnace Road. River Road has an ADT of 2,900 vehicles per day and a posted speed limit of 40 mph east of Spotswood Furnace Road.

Route 620 (Spotswood Furnace Road) is a two-lane local collector with an ADT volume of approximately 2,800 vehicles per day, and a posted speed limit of 35 mph.

Existing Traffic Volumes

The AM peak hour (7:00 to 9:00 AM) and PM peak hour (4:00 to 6:00 PM) turning movement counts were conducted by Technical Traffic Services, LLC at the following intersection during the week of November 28:

Spotswood Furnace Road at River Road

Figure 2 shows the existing 2016 traffic volumes and the count data is enclosed.

Background Traffic Growth

The existing peak hour traffic volumes were grown by an annual rate of 2.0% for two years to estimate the 2018 no-build traffic volumes. The no-build (2018) traffic volumes are shown in Figure 2.

Site Traffic Distribution

The following site traffic distribution was applied based on a review of the existing traffic volumes, the adjacent roadway network, and engineering judgment:

- 40% to / from the east on River Road
- 40% to / from the south on Spotswood Furnace Road
- 20% to / from the west on River Road

Figure 3 shows the site trip distribution and site trip assignment. Figure 4 shows the projected build (2018) AM and PM peak hour traffic volumes.

VDOT Turn Lane Warrant Analysis

The projected build-out AM and PM peak hour traffic volumes at the proposed driveway were compared to the turn lane warrants in the Virginia Department of Transportation (VDOT) Access Management Design Standards for Entrances and Intersections:

- An eastbound left-turn lane on River Road is <u>not</u> warranted
- A westbound right-turn lane or taper on River Road is not warranted

Figure 4 shows the recommended lanes. The turn lane warrant diagrams are enclosed for reference.



Intersection Spacing Standards

VDOT requires at least 335 feet of separation between full-movement intersections on two-lane Minor Collector roadways posted 35 mph. The proposed neighborhood driveway on River Road is approximately 575 feet east of Trench Court, which exceeds VDOT's minimum intersection spacing standards.

Traffic Capacity Analysis

Traffic capacity analysis was performed using Synchro 9.1, which is a comprehensive software package that allows the user to model signalized and unsignalized intersections to determine levels-of-service based on the thresholds specified in the 2010 Highway Capacity Manual (HCM).

Table 2 summarizes the capacity analysis results for the unsignalized intersection of River Road at Spotswood Furnace Road, and all of the Synchro outputs are enclosed for reference.

Table 2
Level-of-Service Summary
Spotswood Furnace Road at River Road (Unsignalized – 4-way stop)

		A	M PEAK	HOUR	PI	M PEAK H	IOUR
CONDITION	GROUP CROUP	Lane LOS	Queue (ft)	Overall LOS (Delay)	Lane LOS	Queue (ft)	Overall LOS (Delay)
	EBL/T/R	В	55		A	8	
Existing 2016	WBL/T/R	A	8	A	A	45	A
	NBL/T/R	A	8	(9.6 sec)	A	8	(9.2 sec)
raffic Conditions	SBL/T/R	A	5	(3.0 500)	A	0	(>12 500)
	EBL/T/R	В	60		A	8	
No-Build 2018	WBL/T/R	Α	8	A	A	48	A
Traffic Conditions	NBL/T/R	Α	10	(9.8 sec)	A	8	(9.3 sec)
	SBL/T/R	A	5	(5.0 500)	Α	0	(>15 500)
	EBL/T/R	В	68		A	10	
Build 2018	WBL/T/R	Α	10	В	В	53	A
Traffic Conditions	NBL/T/R	Α	10	(10.3 sec)	A	10	(9.6 sec
	SBL/T/R	Α	5	(10.5 500)	A	0	(5.0 500

The capacity analysis indicates that the intersection currently operates at LOS A during the AM and PM peak hours. Under no-build conditions, the intersection is expected to continue to operate at LOS A during the AM and PM peak hours. Under build conditions, the intersection is expected to operate at LOS B during the AM peak hour and LOS A during the PM peak hour.

No improvements are warranted or recommended at this intersection.

Table 3 summarizes the capacity analysis results for the proposed unsignalized intersection of River Road at Neighborhood Driveway, and all of the Synchro outputs are enclosed for reference.

Table 3
Level-of-Service Summary
River Road at Neighborhood Driveway (Unsignalized)

		Al	M PEAK H	OUR	PM PEAK HOUR			
CONDITION	LANE GROUP	Lane LOS	Queue (ft)	Overall LOS (Delay)	Lane LOS	Queue (ft)	Overall LOS (Delay)	
Build 2018 Traffic Conditions	Build 2018 EBL/T ² A 0		N/A ³	A B	0 - 3	N/A ³		

1. Level of service for minor approach

2. Level of service for major street left-turn movement

HCM methodology does not provide lane group or overall LOS, delay, and queue lengths for major street through
movements or right turns at unsignalized intersections.

The capacity analysis indicates that the minor street left-turn movement is projected to operate with short delays (less than 25 seconds) during the AM and PM peak hours at build-out of the neighborhood, and the queue lengths are expected to be one vehicle or less. No improvements are warranted or recommended at this intersection.

Sight Distance Evaluation

The sight distance for drivers exiting the proposed neighborhood driveway were measured in the field. Table 4 summarizes the sight distance requirements for driveways on two-lane roadways according to the VDOT Road Design Manual. Table 4 also summarizes the sight distance requirements according to A Policy on Geometric Design of Highways and Streets, which is published by the American Association of State Highway and Transportation Officials (AASHTO).

Table 4
Sight Distance Requirements
(From Exhibits 9-55 and 9-58 in the 2004 AASHTO Green Book)

Design Speed	Turn	VDOT Minimum Sight Distance	AASHTO Minimum Sight Distance	Actual Sight Distance
River Road:	Left	390 feet	445 feet	550 feet
40 mph	Right	390 feet	385 feet	700 feet

At the proposed driveway location on River Road, drivers exiting the site are able to see approximately 550 feet to the west, and 700 feet to the east, which exceeds both the VDOT Road Design Manual and the AASHTO minimum sight distance requirements in both directions.

Enclosed for reference are photos taken at the proposed driveway location looking in both directions.



Mr. Leon Hughes, AICP Page 5 of 5

Collision Data Analysis

Collision data was collected from the Virginia Department of Motor Vehicles (DMV) from January 1, 2013 to December 31, 2015 within 500 feet of the intersection of Spotswood Furnace Road at River Road. In the past three years, there have been only 5 reported collisions within 500 feet of the study intersection, with zero fatalities, and 3 collisions that resulted in an injury. The collision data does not indicate an unusual collision pattern that requires mitigation.

Recommendations

Both study intersections are projected to operate acceptably at build-out of the proposed neighborhood and no off-site roadway improvements are warranted or recommended.

We appreciate your attention to this matter. Please contact me at (804) 217-8560 if you have any questions about this report.

Sincerely yours,

Ramey Kemp & Associates, Inc.

Carl Hultgren, P.E., PTOE Regional Manager

Enclosures: Figures, VDOT turn lane warrant diagrams, Sight distance photos, Collision data, Synchro output

Copy to: Mr. John Rayl

Mr. Justin Franklin, P.E., Fairbanks & Franklin

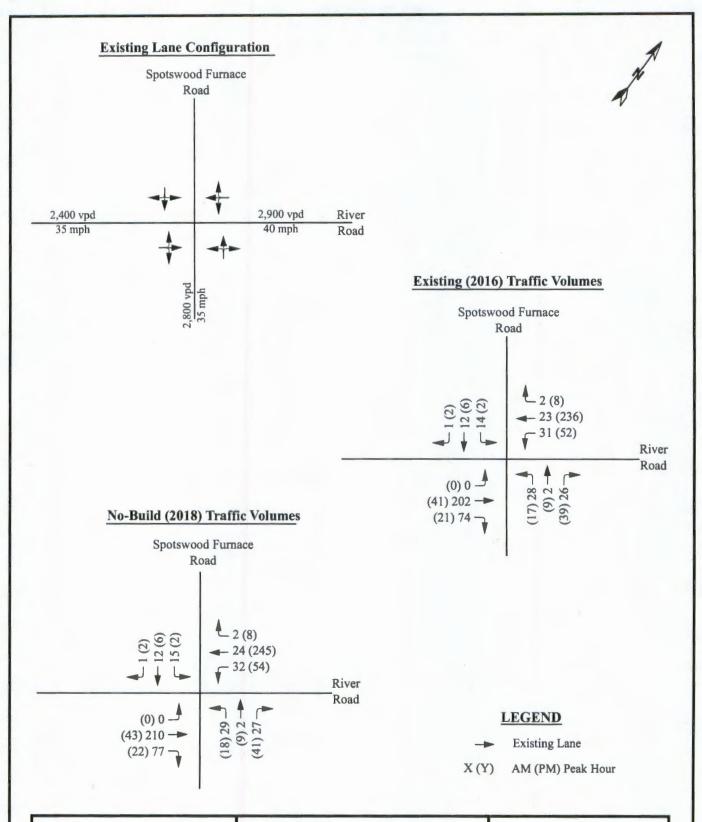




River Road Residential Spotsylvania County, Virginia Site Location and Study Intersections

Scale: Not to Scale

Figure 1

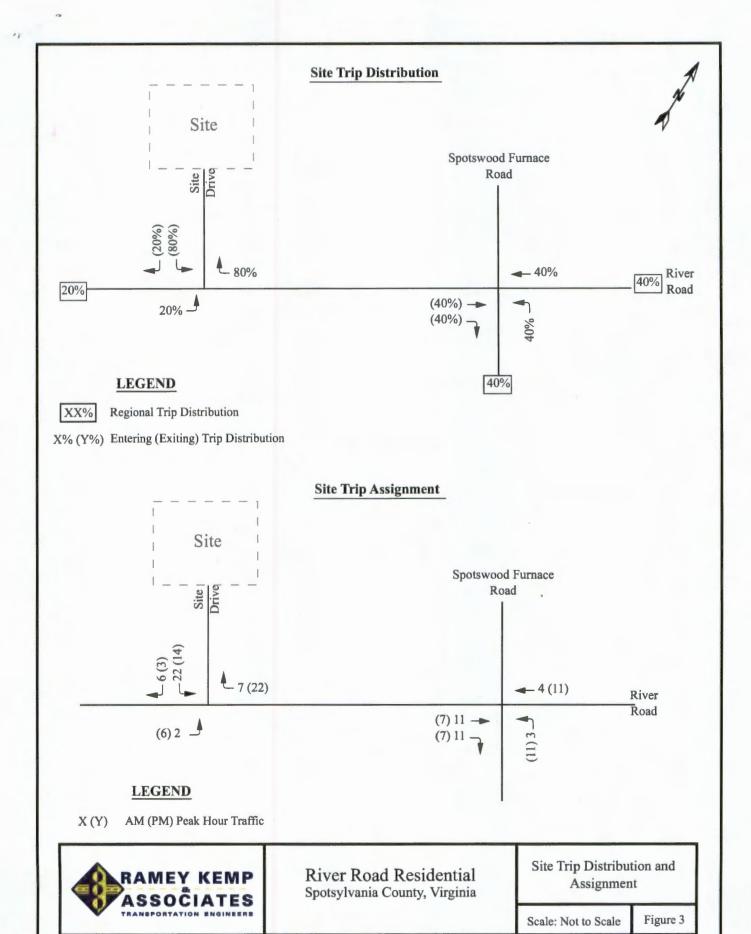




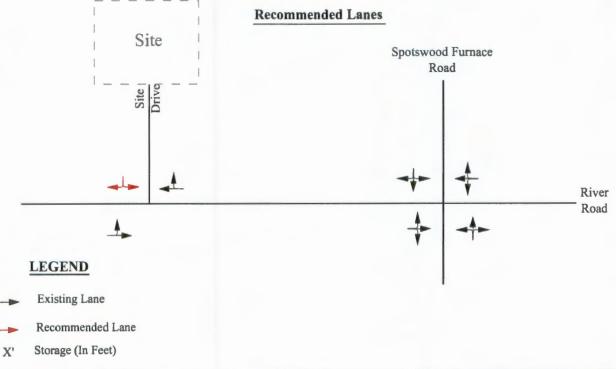
River Road Residential Spotsylvania County, Virginia Existing Lane Configuration, Existing (2016) Traffic Volumes, No-Build (2018) Traffic Volumes

Scale: Not to Scale

Figure 2



Build (2018) Traffic Volumes Site Spotswood Furnace Road _ 28 (256) 7 (22) 32 (54) _ 54 (265) River Road (6) 2 (0) 0 (65) 287 -(50) 221 -(29) 88 -**LEGEND** X(Y)AM (PM) Peak Hour





River Road Residential Spotsylvania County, Virginia Build (2018) Volumes and Recommended Lanes

Scale: Not to Scale

Figure 4

WARRANT FOR LEFT-TURN STORAGE LANES ON TWO-LANE HIGHWAY

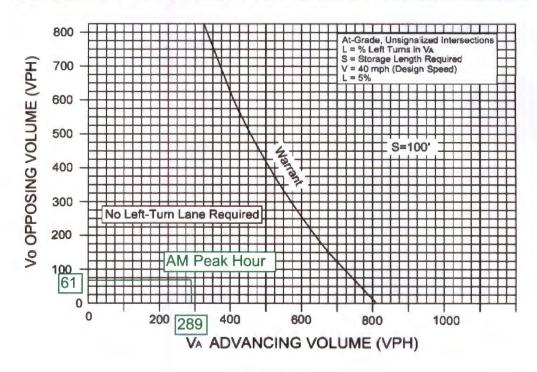


FIGURE 3-5

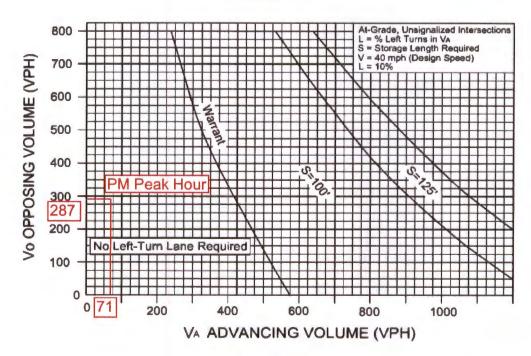
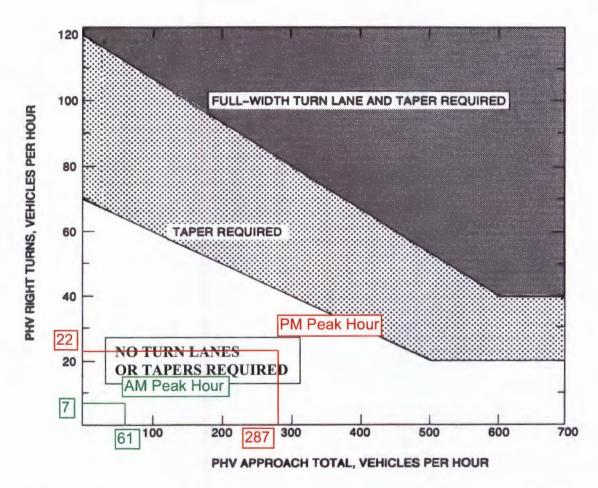


FIGURE 3-6



Appropriate Radius required at all Intersections and Entrances (Commercial or Private).

LEGEND

PHV - Peak Hour Volume (also Design Hourly Volume equivalent)

Adjustment for Right Turns

For posted speeds at or under 45 mph, PHV right turns > 40, and PHV total < 300.

Adjusted right turns = PHV Right Turns - 20

If PHV is not known use formula: PHV = ADT x K x D

K = the percent of AADT occurring in the peak hour

D = the percent of traffic in the peak direction of flow

Note: An average of 11% for K x D will suffice.

When right turn facilities are warranted, see Figure 3-1 for design criteria."

FIGURE 3-26 WARRANTS FOR RIGHT TURN TREATMENT (2-LANE HIGHWAY)

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Exhibit 1 - Looking west on River Road at proposed driveway location

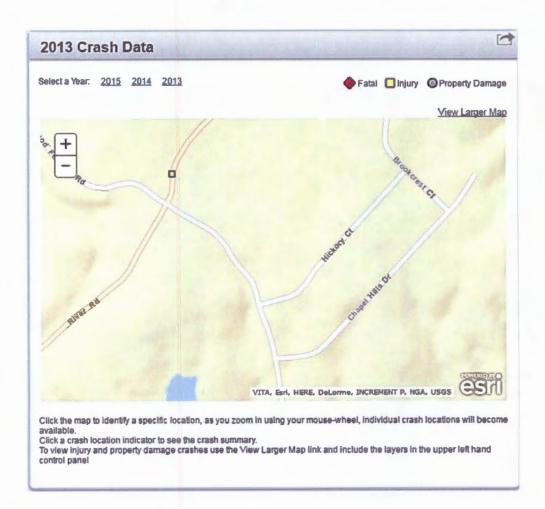


Exhibit 2 - Looking east on River Road at proposed driveway location









Intersection												
Intersection Delay, s/veh	9.6											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Lane Configurations			4				4				4	
Traffic Vol, veh/h	0	1	202	74	0	31	23	2	0	28	2	26
Future Vol, veh/h	0	1	202	74	0	31	23	2	0	28	2	26
Peak Hour Factor	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	1
Mymt Flow	0	1	273	100	0	42	31	3	0	38	3	35
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		10.3				8.2				8.4		
HCM LOS		В				Α				Α		
Lane		NBLn1	EBI n1	WBLn1	SBLn1							
Vol Left, %		50%	0%	55%	52%							
VOI Leit, 76				33 /0	32 /0							
Val Thru %		10/	720/	110/	110%							
Vol Thru, %		4%	73%	41%	44%							
Vol Right, %		46%	27%	4%	4%							
Vol Right, % Sign Control		46% Stop	27% Stop	4% Stop	4% Stop							
Vol Right, % Sign Control Traffic Vol by Lane		46% Stop 56	27% Stop 277	4% Stop 56	4% Stop 27							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol		46% Stop 56 28	27% Stop 277	4% Stop 56 31	4% Stop 27 14							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		46% Stop 56 28 2	27% Stop 277 1 202	4% Stop 56 31 23	4% Stop 27 14 12							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		46% Stop 56 28 2 26	27% Stop 277 1 202 74	4% Stop 56 31 23 2	4% Stop 27 14 12							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		46% Stop 56 28 2 26 76	27% Stop 277 1 202 74 374	4% Stop 56 31 23 2 76	4% Stop 27 14 12 1 36				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		46% Stop 56 28 2 26 76	27% Stop 277 1 202 74 374	4% Stop 56 31 23 2 76	4% Stop 27 14 12 1 36				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		46% Stop 56 28 2 26 76 1 0.101	27% Stop 277 1 202 74 374 1 0.43	4% Stop 56 31 23 2 76 1 0.098	4% Stop 27 14 12 1 36 1 0.052				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		46% Stop 56 28 2 26 76 1 0.101 4.788	27% Stop 277 1 202 74 374 1 0.43 4.14	4% Stop 56 31 23 2 76 1 0.098 4.682	4% Stop 27 14 12 1 36 1 0.052 5.102				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		46% Stop 56 28 2 26 76 1 0.101 4.788 Yes	27% Stop 277 1 202 74 374 1 0.43 4.14 Yes	4% Stop 56 31 23 2 76 1 0.098 4.682 Yes	4% Stop 27 14 12 1 36 1 0.052 5.102 Yes				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		46% Stop 56 28 2 26 76 1 0.101 4.788 Yes 748	27% Stop 277 1 202 74 374 1 0.43 4.14 Yes 872	4% Stop 56 31 23 2 76 1 0.098 4.682 Yes 766	4% Stop 27 14 12 1 36 1 0.052 5.102 Yes 702				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		46% Stop 56 28 2 26 76 1 0.101 4.788 Yes 748 2.817	27% Stop 277 1 202 74 374 1 0.43 4.14 Yes 872 2.159	4% Stop 56 31 23 2 76 1 0.098 4.682 Yes 766 2.709	4% Stop 27 14 12 1 36 1 0.052 5.102 Yes 702 3.134				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		46% Stop 56 28 2 26 76 1 0.101 4.788 Yes 748 2.817 0.102	27% Stop 277 1 202 74 374 1 0.43 4.14 Yes 872 2.159 0.429	4% Stop 56 31 23 2 76 1 0.098 4.682 Yes 766 2.709 0.099	4% Stop 27 14 12 1 36 1 0.052 5.102 Yes 702 3.134 0.051				7			
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		46% Stop 56 28 2 26 76 1 0.101 4.788 Yes 748 2.817	27% Stop 277 1 202 74 374 1 0.43 4.14 Yes 872 2.159	4% Stop 56 31 23 2 76 1 0.098 4.682 Yes 766 2.709	4% Stop 27 14 12 1 36 1 0.052 5.102 Yes 702 3.134				7			

Movement	SBU	SBL	SBT	SBR	
Lane Configurations			4		
Traffic Vol, veh/h	0	14	12	1	
Future Vol, veh/h	0	14	12	1	
Peak Hour Factor	0.92	0.74	0.74	0.74	
Heavy Vehicles, %	2	2	2	2	
Mvmt Flow	0	19	16	1	
Number of Lanes	0	0	. 1	0	
Approach		SB			
Opposing Approach		NB	1		
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8.4			
HCM LOS		Α			

Intersection Delay, s/veh	9.2							* ***				
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBF
Lane Configurations			4				4				4	
Traffic Vol, veh/h	0	1	41	21	0	52	236	8	0	17	9	39
Future Vol, veh/h	0	1	41	21	0	52	236	8	0	17	9	39
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.9
leavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	0	1	46	23	0	58	262	9	0	19	10	43
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	(
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.7				9.8				8		
HCM LOS		Α				Α				Α		
Lane		NBLn1	FRI n1	WBLn1	SBLn1							
Vol Left, %		26%	2%	18%	20%							
VOI LOIL, /U												
Vol Thru %												
Vol Thru, %		14%	65%	80%	60%							
Vol Right, %		14% 60%	65% 33%	80% 3%	60% 20%							
Vol Right, % Sign Control		14% 60% Stop	65% 33% Stop	80% 3% Stop	60% 20% Stop							
Vol Right, % Sign Control Traffic Vol by Lane		14% 60% Stop 65	65% 33% Stop 63	80% 3% Stop 296	60% 20% Stop 10							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol		14% 60% Stop 65 17	65% 33% Stop 63 1	80% 3% Stop 296 52	60% 20% Stop 10 2							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol		14% 60% Stop 65 17 9	65% 33% Stop 63 1	80% 3% Stop 296 52 236	60% 20% Stop 10 2 6							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol		14% 60% Stop 65 17 9	65% 33% Stop 63 1 41 21	80% 3% Stop 296 52 236 8	60% 20% Stop 10 2 6							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate		14% 60% Stop 65 17 9 39 72	65% 33% Stop 63 1 41 21 70	80% 3% Stop 296 52 236 8 329	60% 20% Stop 10 2 6 2							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp		14% 60% Stop 65 17 9 39 72	65% 33% Stop 63 1 41 21 70	80% 3% Stop 296 52 236 8 329	60% 20% Stop 10 2 6 2 11							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X)		14% 60% Stop 65 17 9 39 72 1 0.091	65% 33% Stop 63 1 41 21 70 1 0.083	80% 3% Stop 296 52 236 8 329 1 0.379	60% 20% Stop 10 2 6 2 11 1 0.015							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd)		14% 60% Stop 65 17 9 39 72 1 0.091 4.518	65% 33% Stop 63 1 41 21 70 1 0.083 4.266	80% 3% Stop 296 52 236 8 329 1 0.379 4.151	60% 20% Stop 10 2 6 2 11 1 0.015 4.825							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		14% 60% Stop 65 17 9 39 72 1 0.091 4.518 Yes	65% 33% Stop 63 1 41 21 70 1 0.083 4.266 Yes	80% 3% Stop 296 52 236 8 329 1 0.379 4.151 Yes	60% 20% Stop 10 2 6 2 11 1 0.015 4.825 Yes							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap		14% 60% Stop 65 17 9 39 72 1 0.091 4.518 Yes 797	65% 33% Stop 63 1 41 21 70 1 0.083 4.266 Yes 843	80% 3% Stop 296 52 236 8 329 1 0.379 4.151 Yes 855	60% 20% Stop 10 2 6 2 11 1 0.015 4.825 Yes 745							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time		14% 60% Stop 65 17 9 39 72 1 0.091 4.518 Yes 797 2.524	65% 33% Stop 63 1 41 21 70 1 0.083 4.266 Yes 843 2.277	80% 3% Stop 296 52 236 8 329 1 0.379 4.151 Yes 855 2.233	60% 20% Stop 10 2 6 2 11 1 0.015 4.825 Yes 745 2.834							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N Cap Service Time HCM Lane V/C Ratio		14% 60% Stop 65 17 9 39 72 1 0.091 4.518 Yes 797 2.524 0.09	65% 33% Stop 63 1 41 21 70 1 0.083 4.266 Yes 843 2.277 0.083	80% 3% Stop 296 52 236 8 329 1 0.379 4.151 Yes 855 2.233 0.385	60% 20% Stop 10 2 6 2 11 1 0.015 4.825 Yes 745 2.834 0.015							
Vol Right, % Sign Control Traffic Vol by Lane LT Vol Through Vol RT Vol Lane Flow Rate Geometry Grp Degree of Util (X) Departure Headway (Hd) Convergence, Y/N		14% 60% Stop 65 17 9 39 72 1 0.091 4.518 Yes 797 2.524	65% 33% Stop 63 1 41 21 70 1 0.083 4.266 Yes 843 2.277	80% 3% Stop 296 52 236 8 329 1 0.379 4.151 Yes 855 2.233	60% 20% Stop 10 2 6 2 11 1 0.015 4.825 Yes 745 2.834							

	0.011	0.001		000	
Movement	SBU	SBL	SBT	SBR	
Lane Configurations			4		
Traffic Vol, veh/h	0	2	6	2	
Future Vol, veh/h	0	2	6	2	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	2	2	2	
Mvmt Flow	0	2	7	2	
Number of Lanes	0	0	1	0	
Approach		SB			
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		7.9			
HCM LOS		Α			

Intersection												
Intersection Delay, s/veh	9.8											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations			4				4				4	
Traffic Vol, veh/h	0	1	210	77	0	32	24	2	0	29	2	27
Future Vol, veh/h	0	1	210	77	0	32	24	2	0	29	2	27
Peak Hour Factor	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1	284	104	0	43	32	3	0	39	3	36
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		10.5				8.3				8.4		
HCM LOS		В				Α				Α		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		50%	0%	55%	54%							
Vol Thru, %		3%	73%	41%	43%							
Vol Right, %		47%	27%	3%	4%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		58	288	58	28							
LT Vol		29	1	32	15							
Through Vol		2	210	24	12							
RT Vol		27	77	2	1							
Lane Flow Rate		78	389	78	38							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.105	0.449	0.103	0.054							
Departure Headway (Hd)		4.83	4.156	4.712	5.152							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Cap		741	868	761	694							
Service Time		2.864	2.175	2.74	3.188							
HCM Lane V/C Ratio		0.105	0.448	0.102	0.055							
HCM Control Delay		8.4	10.5	8.3	8.5							
HCM Lane LOS		Α	В	Α	Α							
HCM 95th-tile Q		0.4	2.4	0.3	0.2							

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Movement	SBU	SBL	SBT	SBR	
Lane Configurations			4		
Traffic Vol, veh/h	0	15	12	1	
Future Vol, veh/h	0	15	12	1	
Peak Hour Factor	0.92	0.74	0.74	0.74	
Heavy Vehicles, %	2	2	2	2	
Mymt Flow	0	20	16	1	
Number of Lanes	0	0	1	0	
Approach		SB			
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8.5			
HCM LOS		Α			

Intersection												
Intersection Delay, s/veh	9.3											
Intersection LOS	Α											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations			4				4				4	
Traffic Vol, veh/h	0	1	43	22	0	54	245	8	0	18	9	41
Future Vol, veh/h	0	1	43	22	0	54	245	8	0	18	9	41
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	0	1	48	24	0	60	272	9	0	20	10	46
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.7				10				8		
HCM LOS		Α				Α				Α		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		26%	2%	18%	20%							
Vol Thru, %		13%	65%	80%	60%							
Vol Right, %		60%	33%	3%	20%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		68	66	307	10							
LT Vol		18	1	54	2							
Through Vol		9	43	245	6							
RT Vol		41	22	8	2							
Lane Flow Rate		76	73	341	11							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.096	0.087	0.394	0.015							
Departure Headway (Hd)		4.554	4.288	4.16	4.867							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		791	838	853	738							
Service Time		2.558	2.3	2.247	2.876							
HCM Lane V/C Ratio		0.096	0.087	0.4	0.015							
HCM Control Delay		8	7.7	10	8							
HCM Lane LOS		Α	Α	Α	Α							
HCM 95th-tile Q		0.3	0.3	1.9	0							

Movement	SBU	SBL	SBT	SBR	
Lane Configurations			4		
Traffic Vol, veh/h	0	2	6	2	
Future Vol, veh/h	0	2	6	2	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	2	2	2	
Mvmt Flow	0	2	7	2	
Number of Lanes	0	0	1	0	
Approach		SB			
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8			
HCM LOS		Α			

Intersection												
Intersection Delay, s/veh Intersection LOS	10.3 B											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	<u>NBT</u>	NBR
Lane Configurations			4				4				4	
Traffic Vol, veh/h	0	1	221	88	0	32	28	2	0	32	2	27
Future Vol, veh/h	0	1	221	88	0	32	28	2	0	32	2	27
Peak Hour Factor	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74	0.92	0.74	0.74	0.74
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1	299	119	0	43	38	3	0	43	3	36
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		11.1				8.4				8.6		
HCM LOS		В				Α				Α		
		NBLn1	CDI -1	WBLn1	SBLn1							
Lane		52%	0%	52%	54%							
Vol Left, %		3%	71%	52% 45%	43%							
Vol Thru, %		3% 44%	28%	3%	43% 4%							
Vol Right, %			Stop									
Sign Control Traffic Val by Lane		Stop 61	310	Stop 62	Stop 28							
Traffic Vol by Lane LT Vol		32	1	32	15							
		2	221	28	12							
Through Vol RT Vol		27	88	20	1							
Lane Flow Rate		82	419	84	38							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.113	0.485	0.111	0.055							
Departure Headway (Hd)		4.925	4.168	4.753	5.237							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Cap		726	867	753	682							
Service Time		2.966	2.19	2.786	3.281							
HCM Lane V/C Ratio		0.113	0.483	0.112	0.056							
HCM Control Delay		8.6	11.1	8.4	8.6							
i ioivi oolilloi belay		0.0	11.1									
HCM Lane LOS		Α	В	Α	Α							

Intersection							
Intersection Delay, s/veh Intersection LOS							
Movement	SBU	SBL	SBT	SBR			
Lane Configurations			4				
Traffic Vol, veh/h	0	15	12	1			
Future Vol, veh/h	0	15	12	1			
Peak Hour Factor	0.92	0.74	0.74	0.74			
Heavy Vehicles, %	2	2	2	2			
Mvmt Flow	0	20	16	1			
Number of Lanes	0	0	1	0			
Approach		SB					
Opposing Approach		NB					
Opposing Lanes		1					
Conflicting Approach Left		WB					
Conflicting Lanes Left		1					
Conflicting Approach Right		EB					
Conflicting Lanes Right		1					
HCM Control Delay		8.6					
HCM LOS		Α					

ntersection nt Delay, s/veh 0	.9								
Movement	EBL	EBT			WBT	WBR	SBL	SBR	
ane Configurations	LUL	4			1	11011	Y		
Traffic Vol, veh/h	2	287			54	7	22	6	
Future Vol, veh/h	2	287			54	7	22	6	
Conflicting Peds, #/hr	0	0			0	0	0	0	
Sign Control	Free	Free			Free	Free	Stop	Stop	
RT Channelized	1100	None			-	None	0.00	None	
Storage Length		140110			_	-	0	-	
/eh in Median Storage, #		0			0	_	0		
Grade, %		0			0	-	0		
Peak Hour Factor	92	92			92	92	92	92	
Heavy Vehicles, %	2	2			2	2	2	2	
Mymt Flow	2	312			59	8	24	7	
MAINT I IOW	2	012			00	Ü	2.1		
Major/Minor	Major1			N	lajor2		Minor2		
Conflicting Flow All	66	0			-	0	378	62	
Stage 1	-	-			-		62		
Stage 2	-	-			-	-	316		
Critical Hdwy	4.12	-			-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-			-	-	5.42		
Critical Hdwy Stg 2	-	-			-	-	5.42		
Follow-up Hdwy	2.218	-			-	-	3.518	3.318	
Pot Cap-1 Maneuver	1536	-			-		624	1003	
Stage 1	-	-			-	-	961		
Stage 2	-	-			-	-	739		
Platoon blocked, %		-			-	-			
Mov Cap-1 Maneuver	1536	-			-	-	623	1003	
Mov Cap-2 Maneuver	-	-			-	-	623		
Stage 1	-	-			-	-	961		
Stage 2		-			-	-	738		
Approach	EB				WB		SB		
HCM Control Delay, s	0.1				0		10.6		
HCM LOS	0.1				U		В		
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1					
Capacity (veh/h)	1536	-	-	- 678					
HCM Lane V/C Ratio	0.001	_	-	- 0.045					
HCM Control Delay (s)	7.3	0	-	- 10.6					
HCM Lane LOS	Α.	A		- B					
HCM 95th %tile Q(veh)	0	, ,		- 0.1					

Intersection												
Intersection Delay, s/veh Intersection LOS	9.6 A											
Movement	EBU	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBU	NBL	NBT	NBR
Lane Configurations			e‡s				4				4	
Traffic Vol, veh/h	0	1	50	29	0	54	256	8	0	29	9	41
Future Vol, veh/h	0	1	50	29	0	54	256	8	0	29	9	41
Peak Hour Factor	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90	0.92	0.90	0.90	0.90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1	56	32	0	60	284	9	0	32	10	46
Number of Lanes	0	0	1	0	0	0	1	0	0	0	1	0
Approach		EB				WB				NB		
Opposing Approach		WB				EB				SB		
Opposing Lanes		1				1				1		
Conflicting Approach Left		SB				NB				EB		
Conflicting Lanes Left		1				1				1		
Conflicting Approach Right		NB				SB				WB		
Conflicting Lanes Right		1				1				1		
HCM Control Delay		7.9				10.4				8.3		
HCM LOS		Α				В				Α		
Lane		NBLn1	EBLn1	WBLn1	SBLn1							
Vol Left, %		37%	1%	17%	20%							
Vol Thru, %		11%	62%	81%	60%							
Vol Right, %		52%	36%	3%	20%							
Sign Control		Stop	Stop	Stop	Stop							
Traffic Vol by Lane		79	80	318	10							
LT Vol		29	1	54	2							
Through Vol		9	50	256	6							
RT Vol		41	29	8	2							
Lane Flow Rate		88	89	353	11							
Geometry Grp		1	1	1	1							
Degree of Util (X)		0.114	0.107	0.422	0.015							
Departure Headway (Hd)		4.69	4.324	4.3	4.953							
Convergence, Y/N		Yes	Yes	Yes	Yes							
Сар		765	830	841	722							
Service Time		2.716	2.345	2.3	2.985							
HCM Lane V/C Ratio		0.115	0.107	0.42	0.015							
HCM Control Delay		8.3	7.9	10.4	8.1							
HCM Lane LOS		Α	Α	В	Α							
HCM 95th-tile Q		0.4	0.4	2.1	0							

Movement	SBU	SBL	SBT	SBR	
Lane Configurations			4		
Traffic Vol, veh/h	0	2	6	2	
Future Vol, veh/h	0	2	6	2	
Peak Hour Factor	0.92	0.90	0.90	0.90	
Heavy Vehicles, %	2	2	2	2	
Mymt Flow	0	2	7	2	
Number of Lanes	0	0	1	0	
Approach		SB			
Opposing Approach		NB			
Opposing Lanes		1			
Conflicting Approach Left		WB			
Conflicting Lanes Left		1			
Conflicting Approach Right		EB			
Conflicting Lanes Right		1			
HCM Control Delay		8.1			
HCM LOS		Α			

ntersection										
nt Delay, s/veh	0.6									
Movement	EBL	EBT			WBT	WBR	SE		SBR	
Lane Configurations		ब			1		1	Υf		
Traffic Vol, veh/h	6	65			265	22		14	3	
Future Vol, veh/h	6	65			265	22		14	3	
Conflicting Peds, #/hr	0	0			0	0		0	0	
Sign Control	Free	Free			Free	Free	Sto	ор	Stop	
RT Channelized	-	None			-	None		-	None	
Storage Length	-	-			-	-		0	-	
Veh in Median Storage, #	-	0			0	-		0		
Grade, %	-	0			0	-		0	-	
Peak Hour Factor	92	92			92	92	9	92	92	
Heavy Vehicles, %	2	2			2	2		2	2	
Vivmt Flow	7	71			288	24	,	15	3	
Major/Minor	Major1				Major2		Mino	r2		
Conflicting Flow All	312	0			-	0	38	84	300	
Stage 1	-	_			-	-		00	-	
Stage 2		-				_		84		
Critical Hdwy	4.12				_		6.4		6.22	
Critical Hdwy Stg 1		_				_	5.4		-	
Critical Hdwy Stg 2		_				-	5.4			
Follow-up Hdwy	2.218	_			_	_	3.5		3.318	
Pot Cap-1 Maneuver	1248	_			_	_		19	740	
Stage 1	1210	-			_	_		52		
Stage 2		-			_			39	_	
Platoon blocked, %		-				_		00		
Mov Cap-1 Maneuver	1248				_	_	6	15	740	
Mov Cap-1 Maneuver	1240							15	-	
Stage 1								52		
Stage 2								33		
Approach	EB				WB			SB		
HCM Control Delay, s	0.7				0		10			
HCM LOS	0.7				Ü		10	В		
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1						
Capacity (veh/h)	1248	-	-	- 634						
HCM Lane V/C Ratio	0.005		-	- 0.029						
HCM Control Delay (s)	7.9	0		- 10.8						
HCM Lane LOS	Α	A	-	- B						
HCM 95th %tile Q(veh)	0			- 0.1						